

# On a Large Deflection of Clamped Square Plate under a Lateral Load.

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## Introduction.

1. It is known that two fundamental assumptions are made usually in the theory of elasticity : small deflections and linear stress-strain relations. Under these assumptions the problem of stress distribution in elastic systems is governed by linear partial differential equations. However, if we give up one or both of these assumptions, we obtain non-linear equations. The Kirchhoff theory of the bending of elastic plates is also based on the above same assumptions and that thickness is small compared to the area dimensions. The first assumption allows one to neglect the extensions of the middle surface which accompanies bending, while the third assumption permits one to consider that the straight lines initially normal to the middle surface remain straight and normal to that surface after bending.

When the deflection of the plate is of the same order as the thickness, the effect of the middle surface extension becomes important and must be in considerations. And then the differential equations governing the deformation of the plate becomes non-linear and exact solutions are feasible only in a few simple cases. We must then adopt a method of systematic successive approximations such as the method of finite differences or the Ritz method.

Hencky solved approximately by using the method of finite differences the problem of the bending of the uniformly laterally loaded rectangular plate which has negligible flexural rigidity, no initial tension and the clamped four edges. The problem of a square plate with simply supported edges and no edge traction is solved by Kaiser by the same method. S. Way solved, by the Ritz energy method, the problem of a uniformly laterally loaded clamped rectangular plate with large deflection.

The problem to be treated here is such that the distribution of lateral load over a square plate is symmetrical to its center and the plate is clamped. And Ritz method will be applied. It is very interesting and important both theoretically and practically to solve these problems.

## 2. Fundamental Equations and Notations.

Let the plate be situated with respect to the coordinates axes as shown in Fig 1.

The following notations will be used :

$X, Y, Z$  = coordinates of a point

$2a$  = length of side of the plate

$2\delta$  = length of side of the square part on which the load situates

$h$  = thickness of the plate

$u, v, w$  = displacements of points in the middle surface in directions  $X, Y, Z$

$E, \mu$  = Young's modulus and Poisson's ratio

$G$  = shear modulus of elasticity

$D = \frac{Eh^3}{12(1-\mu^2)}$ , the flexural rigidity

$P$  = lateral load per unit of area

$U$  = total energy of the plate and load

$\sigma'_x, \sigma'_y, \tau'_{xy}$  = stresses in the middle surface

$\sigma''_x, \sigma''_y, \tau''_{xy}$  = bending stresses and shearing

stress at the surface  $Z = -\frac{h}{2}$

$Q_x, Q_y$  = vertical shearing forces per unit of length on plate

$X = \text{const}, Y = \text{const}$

$e'_x, e'_y, \gamma'_{xy}$  = extensions and shear in the middle surface

$m_x, m_y, m_{xy}$  = bending and twisting moments per unit of length.

Then the following relations exists:

$$\left. \begin{aligned} m_x &= -D \left( \frac{\partial^2 w}{\partial X^2} + \mu \frac{\partial^2 w}{\partial Y^2} \right), & m_y &= -D \left( \frac{\partial^2 w}{\partial Y^2} + \mu \frac{\partial^2 w}{\partial X^2} \right), \\ m_{xy} &= -(1-\mu) D \frac{\partial^2 w}{\partial X \partial Y}, \end{aligned} \right\} \quad (1)$$

while the bending stresses on the upper surface  $Z = -\frac{h}{2}$  are give in terms of the moments by :

$$\sigma''_x = -\frac{6}{h^2} m_x; \quad \sigma''_y = -\frac{6}{h^2} m_y; \quad \tau''_{xy} = -\frac{6}{h^2} m_{xy} \quad (2)$$

The vertical shearing forces are

$$Q_x = -D \frac{\partial}{\partial X} (\Delta w); \quad Q_y = -D \frac{\partial}{\partial Y} (\Delta w) \quad (3)$$

in which

$$\Delta w = \frac{\partial^2 w}{\partial X^2} + \frac{\partial^2 w}{\partial Y^2}$$

$e'_x, e'_y$  and  $\gamma'_{xy}$  are

$$e'_x = \frac{\partial u}{\partial X} + \frac{1}{2} \left( \frac{\partial w}{\partial X} \right)^2; \quad e'_y = \frac{\partial v}{\partial Y} + \frac{1}{2} \left( \frac{\partial w}{\partial Y} \right)^2; \quad \gamma'_{xy} = \frac{\partial u}{\partial Y} + \frac{\partial v}{\partial X} + \frac{\partial w}{\partial X} \frac{\partial w}{\partial Y} \quad (4)$$

By Hooke's law,

$$\sigma'_x = \frac{E}{1-\mu^2} (e'_x + \mu e'_y); \quad \sigma'_y = \frac{E}{1-\mu^2} (e'_y + \mu e'_x); \quad \tau_{xy} = G \gamma'_{xy} \quad (5)$$

From the above equations, we can calculate the stresses and moments everywhere in the plate when the displacement  $u, v,$  and  $w$  are known. Three other equations which are necessary to determine the displacements are obtained by considering the equilibrium of an element  $hdXdY$ :

$$\left. \begin{aligned} \frac{\partial \sigma'_x}{\partial X} + \frac{\partial \tau'_{xy}}{\partial Y} &= 0, \\ \frac{\partial \sigma'_y}{\partial Y} + \frac{\partial \tau'_{xy}}{\partial X} &= 0, \end{aligned} \right\} \quad (6)$$

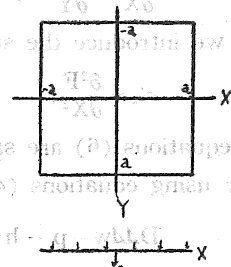


Fig. 1

$$\frac{\partial Q_x}{\partial X} + \frac{\partial Q_y}{\partial Y} + p + h \left\{ \sigma'_x \frac{\partial^2 w}{\partial X^2} + \sigma'_y \frac{\partial^2 w}{\partial Y^2} + 2\tau'_{xy} \frac{\partial^2 w}{\partial X \partial Y} \right\} = 0. \quad (7)$$

If we introduce the stress function  $F$  such as

$$\sigma'_x = \frac{\partial^2 F}{\partial X^2}; \quad \sigma'_y = \frac{\partial^2 F}{\partial Y^2}; \quad \tau'_{xy} = -\frac{\partial^2 F}{\partial X \partial Y}, \quad (8)$$

the equations (6) are satisfied identically.

By using equations (4), (5) and (8) the equation (7) becomes

$$D\Delta\Delta w - p - h \left\{ \frac{\partial^2 F}{\partial Y^2} \cdot \frac{\partial^2 w}{\partial X^2} + \frac{\partial^2 F}{\partial X^2} \cdot \frac{\partial^2 w}{\partial Y^2} - 2 \frac{\partial^2 F}{\partial X \partial Y} \cdot \frac{\partial^2 w}{\partial X \partial Y} \right\} = 0. \quad (9)$$

When the extensions in equations (5) are introduced into the equations (6) and from the three resulting equations  $u$  and  $v$  are eliminated, we obtain the following equation:

$$\Delta\Delta F = E \left\{ \left( \frac{\partial^2 w}{\partial X \partial Y} \right)^2 - \frac{\partial^2 w}{\partial X^2} \cdot \frac{\partial^2 w}{\partial Y^2} \right\}. \quad (10)$$

These equations were given by Kármán. The two equations (9) and (10) in the unknown  $w$  and  $F$  are the non-linear differential equations governing the general problem of the bending of a plate with large deflection. Since these equations are virtually impossible to solve in most cases, we must be had to approximate methods of treatment. To this problem, we apply the Ritz method. In applying the energy method, we need the expression for the total potential energy of the system consisting of the plate and the loads upon it. This will be,

$$U = \frac{D}{2} \iint (\Delta w)^2 dXdY - \iint p w dXdY + \frac{h}{2} \iint (\sigma'_x e'_x + \sigma'_y e'_y + \tau'_{xy} \gamma'_{xy}) dXdY \quad (11)$$

for a clamped plate. The first and third integrals are taken over the entire plate. The second integral is taken the part of the plate upon which the loads situate. The first integral represents the strain energy of bending and twisting, the second the potential energy of the lateral load, and the third the strain energy of stretching.

If the membrane stresses and extensions in (11) are expressed in terms of the derivatives of  $u$ ,  $v$  and  $w$  and make (11) dimensionless, the energy expression becomes:

$$A = \frac{1}{2} \iint (\Delta \zeta)^2 dx dy - \iint q \zeta dx dy + 6 \iint \left[ \xi_x^2 + \zeta_x^2 \xi_x + \eta_y^2 + \zeta_y^2 \eta_y + \left( \frac{\zeta_x^2}{2} + \frac{\zeta_y^2}{2} \right)^2 + 2\mu \left( \xi_x \eta_y + \eta_y \frac{\zeta_x^2}{2} + \xi_x \frac{\zeta_y^2}{2} \right) + \frac{1-\mu}{2} \left( \xi_y^2 + \eta_x^2 + 2\xi_y \eta_x + 2\xi_y \zeta_x \zeta_y + 2\eta_x \zeta_x \zeta_y \right) \right] dx dy. \quad (12)$$

in which the subscripts indicate partial differentiation and in which we put,

$$\left. \begin{aligned} x &= \frac{X}{a}, & y &= \frac{Y}{a}, & z &= \frac{Z}{a}, & \delta &= \frac{d}{a}, \\ \xi &= \frac{ua}{h^2}, & \eta &= \frac{va}{h^2}, & \zeta &= \frac{w}{h}, & A &= \frac{Ua^2}{Dh^2}, & q &= \frac{pa^4}{Dh} \\ s'_x &= \frac{\sigma'_x a^2 (1-\mu^2)}{Eh^2}, & s'_y &= \frac{\sigma'_y a^2 (1-\mu^2)}{Eh^2}, & s'_{xy} &= \frac{\tau'_{xy} a^2}{Gh^2} \\ s''_x &= \frac{\sigma''_x a^2 (1-\mu^2)}{Eh^2}, & s''_{xy} &= \frac{\sigma''_y a^2 (1-\mu^2)}{Eh^2}, & s''_{xy} &= \frac{\tau''_{xy} a^2}{Gh^2} \end{aligned} \right\} \quad (13)$$

As the plate are clamped, the following boundary conditions must be satisfied:

$$\begin{aligned}
(\zeta)_{x=\pm 1} &= (\zeta)_{y=\pm 1} = 0 \\
(\zeta_x)_{x=\pm 1} &= (\zeta_y)_{y=\pm 1} = 0 \\
(\xi)_{x=\pm 1} &= (\xi)_{y=\pm 1} = 0 \\
(\eta)_{x=\pm 1} &= (\eta)_{y=\pm 1} = 0
\end{aligned} \tag{14}$$

These conditions will be satisfied if we assume for  $\xi$ ,  $\eta$ , and  $\zeta$  the following expressions,

$$\xi = x(1-x^2)(1-y^2)(b_{00} + b_{02}y^2 + b_{20}x^2 + b_{22}x^2y^2) \tag{15}$$

$$\eta = y(1-x^2)(1-y^2)(b_{00} + b_{02}x^2 + b_{20}y^2 + b_{22}x^2y^2) \tag{16}$$

$$\zeta = (1-x^2)^2(1-y^2)^2(a_{00} + a_{02}x^2 + a_{20}y^2 + a_{22}x^2y^2) \tag{17}$$

These expressions satisfy the following symmetry conditions:

$$\begin{aligned}
\xi(x,y) &= \xi(x,-y) = -\xi(-x,y) = -\xi(-x,-y) \\
\eta(x,y) &= \eta(-x,y) = -\eta(x,-y) = -\eta(-x,-y)
\end{aligned} \tag{18}$$

$$\xi(x,y) = \eta(y,x)$$

In equation (27),  $a_{00}$  is the only constant and  $a_{02}$ ,  $a_{22}$  provide an additional means of variation of the deflection form.

As the expression of symmetrical lateral load, we make use of the following expression;

$$q = q_{00} + q_{02}(x^2 + y^2) \tag{19}$$

By Ritz method, since the constants are to be determined so as to total energy  $A$  a minimum, we must have

$$\frac{\partial A}{\partial b_{00}} = 0; \quad \frac{\partial A}{\partial b_{02}} = 0; \quad \frac{\partial A}{\partial b_{20}} = 0; \quad \frac{\partial A}{\partial b_{22}} = 0 \tag{20}$$

$$\frac{\partial A}{\partial a_{00}} = 0; \quad \frac{\partial A}{\partial a_{02}} = 0; \quad \frac{\partial A}{\partial a_{22}} = 0 \tag{21}$$

The four equations (20) are linear in the b's and found to be:

$$\begin{aligned}
&(0.513016 - 0.015238 \mu)b_{00} + (0.066032 - 0.015238 \mu)b_{02} + (0.215026 - 0.001693 \mu)b_{20} \\
&+ (0.027332 - 0.005563 \mu)b_{22} + (-0.030022 + 0.157109 \mu)a_{00}^2 + (-0.089645 - 0.004829 \mu)a_{00}a_{02} \\
&+ (-0.004409 - 0.002729 \mu)a_{00}a_{22} + (-0.015536 + 0.011337 \mu)a_{02}^2 + (-0.002277 + 0.000759 \mu)a_{02}a_{22} \\
&- 0.000097 a_{22}^2 = 0.
\end{aligned} \tag{22. 1}$$

$$\begin{aligned}
&(0.066032 - 0.015238 \mu)b_{00} + (0.037007 - 0.015238 \mu)b_{02} + (0.031202 - 0.001693 \mu)b_{20} \\
&+ (0.013787 - 0.005563 \mu)b_{22} + (0.013646 + 0.019105 \mu)a_{00}^2 + (-0.007348 + 0.008608 \mu)a_{00}a_{02} \\
&+ (-0.002471 + 0.001405 \mu)a_{00}a_{22} + (-0.001324 + 0.002293 \mu)a_{02}^2 + (-0.000613 + 0.000242 \mu)a_{02}a_{22} \\
&- 0.000029 a_{22}^2 = 0.
\end{aligned} \tag{22. 2}$$

$$\begin{aligned}
&(0.215026 - 0.001693 \mu)b_{00} + (0.031202 - 0.001693 \mu)b_{02} + (0.169994 - 0.001165 \mu)b_{20} \\
&+ (0.024517 - 0.000813 \mu)b_{22} + (0.047657 + 0.034640 \mu)a_{00}^2 + (-0.001566 - 0.001050 \mu)a_{00}a_{02} \\
&+ (-0.018265 + 0.007768 \mu)a_{00}a_{22} + (-0.004083 + 0.003426 \mu)a_{02}^2 + (-0.000776 + 0.000611 \mu)a_{02}a_{22} \\
&+ (-0.000038 + 0.000089 \mu)a_{22}^2 = 0.
\end{aligned} \tag{22. 3}$$

$$\begin{aligned}
&(0.027332 - 0.005563 \mu)b_{00} + (0.013787 - 0.005563 \mu)b_{02} + (0.024517 - 0.000814 \mu)b_{20} \\
&+ (0.009917 - 0.002338 \mu)b_{22} + (0.007348 + 0.004409 \mu)a_{00}^2 + (0.000468 + 0.003117 \mu)a_{00}a_{02} \\
&+ (-0.000662 + 0.000501 \mu)a_{00}a_{22} + (-0.000146 + 0.000948 \mu)a_{02}^2 + (-0.000236 + 0.000307 \mu)a_{02}a_{22} \\
&+ (-0.000015 + 0.000028 \mu)a_{22}^2 = 0
\end{aligned} \tag{22. 4}$$

The equations (21) become:



$$\begin{aligned}
& 1.114558 a_{00} + 0.202647 a_{02} + 0.011258 a_{22} \\
& + a_{01}[-0.120087 b_{00} + 0.054585 b_{02} + 0.190628 b_{20} + 0.029392 b_{22}] \\
& + a_{02}[-0.179291 b_{00} - 0.014696 b_{02} - 0.036530 b_{20} + 0.000937 b_{22}] \\
& + a_{22}[-0.008818 b_{00} - 0.004942 b_{02} - 0.003133 b_{20} - 0.001324 b_{22}] \\
& + \mu a_{00}[0.600435 b_{00} + 0.076419 b_{02} + 0.138562 b_{20} + 0.017635 b_{22}] \\
& + \mu a_{02}[-0.009657 b_{00} + 0.017215 b_{02} + 0.015536 b_{20} + 0.006234 b_{22}] \\
& + \mu a_{22}[-0.005458 b_{00} + 0.002810 b_{02} - 0.002099 b_{20} + 0.001001 b_{22}] \\
& + 0.785994 a_{00}^3 + 0.079869 a_{00}^2 a_{02} - 0.016480 a_{00}^2 a_{22} + 0.150197 a_{00} a_{02}^2 + 0.001510 a_{00} a_{22}^2 \\
& + 0.014022 a_{00} a_{02} a_{22} + 0.010995 a_{02}^3 + 0.001940 a_{02}^2 a_{22} + 0.000291 a_{02} a_{22}^2 + 0.000007 a_{22}^3 \\
& - \frac{1}{12} \left( \delta - \frac{2}{3} \delta^3 + \frac{1}{5} \delta^5 \right)^2 q_{00} - \frac{1}{6} \left( \delta - \frac{2}{3} \delta^3 + \frac{1}{5} \delta^5 \right) \left( \frac{1}{3} \delta^3 - \frac{2}{5} \delta^5 + \frac{1}{7} \delta^7 \right) q_{02} = 0. \quad (23. 1)
\end{aligned}$$

$$\begin{aligned}
& 0.202647 a_{00} + 0.457255 a_{02} + 0.036372 a_{22} \\
& + a_{00}[-0.179291 b_{00} - 0.014696 b_{02} - 0.036530 b_{20} + 0.000937 b_{22}] \\
& + a_{02}[-0.062143 b_{00} - 0.005297 b_{02} - 0.016332 b_{20} - 0.000585 b_{22}] \\
& + a_{22}[-0.004554 b_{00} - 0.001226 b_{02} - 0.001552 b_{20} - 0.000473 b_{22}] \\
& + \mu a_{00}[-0.009657 b_{00} + 0.017215 b_{02} + 0.015536 b_{20} + 0.006234 b_{22}] \\
& + \mu a_{02}[+0.045348 b_{00} + 0.009173 b_{02} + 0.013702 b_{20} + 0.003792 b_{22}] \\
& + \mu a_{22}[+0.001518 b_{00} + 0.000484 b_{02} + 0.001222 b_{20} + 0.000614 b_{22}] \\
& + 0.026623 a_{00}^3 + 0.150197 a_{00}^2 a_{02} + 0.007011 a_{00}^2 a_{22} + 0.032985 a_{00} a_{02}^2 + 0.003880 a_{00} a_{22}^2 \\
& + 0.009870 a_{02}^3 + 0.000291 a_{00} a_{22}^2 + 0.001945 a_{02}^2 a_{22} + 0.000269 a_{02} a_{22}^2 + 0.000014 a_{22}^3 \\
& - \frac{1}{6} \left( \delta - \frac{2}{3} \delta^3 + \frac{1}{5} \delta^5 \right) \left( \frac{1}{3} \delta^3 - \frac{2}{5} \delta^5 + \frac{1}{7} \delta^7 \right) q_{00} - \frac{1}{6} \left\{ \left( \delta - \frac{2}{3} \delta^3 + \frac{1}{5} \delta^5 \right) \left( \frac{1}{5} \delta^5 - \frac{2}{7} \delta^7 + \frac{1}{9} \delta^9 \right) \right. \\
& \left. + \left( \frac{1}{3} \delta^3 - \frac{2}{5} \delta^5 + \frac{1}{7} \delta^7 \right)^2 \right\} q_{02} = 0. \quad (23. 2)
\end{aligned}$$

$$\begin{aligned}
& 0.011258 a_{00} + 0.036372 a_{02} + 0.009841 a_{22} \\
& + a_{00}[-0.008818 b_{00} - 0.004942 b_{02} - 0.003133 b_{20} + 0.001324 b_{22}] \\
& + a_{02}[-0.004554 b_{00} - 0.001226 b_{02} - 0.001552 b_{20} - 0.000473 b_{22}] \\
& + a_{22}[-0.000388 b_{00} - 0.000114 b_{02} - 0.000152 b_{20} - 0.000061 b_{22}] \\
& + \mu a_{00}[-0.005458 b_{00} + 0.002810 b_{02} - 0.002099 b_{20} + 0.001001 b_{22}] \\
& + \mu a_{02}[0.001518 b_{00} + 0.000484 b_{02} + 0.001222 b_{20} + 0.000614 b_{22}] \\
& + \mu a_{22}[0.000357 b_{20} + 0.000114 b_{22}] \\
& - 0.005493 a_{00}^3 + 0.007011 a_{00}^2 a_{02} + 0.001510 a_{00}^2 a_{22} + 0.001940 a_{00} a_{02}^2 + 0.000582 a_{00} a_{22}^2 \\
& + 0.000020 a_{00} a_{22}^2 + 0.000648 a_{02}^3 + 0.000269 a_{02}^2 a_{22} + 0.000043 a_{02} a_{22}^2 + 0.000003 a_{22}^3 \\
& - \frac{1}{12} \left( \frac{1}{3} \delta^3 - \frac{2}{5} \delta^5 + \frac{1}{7} \delta^7 \right)^2 q_{00} - \frac{1}{6} \left( \frac{\delta^3}{3} - \frac{2}{5} \delta^5 + \frac{1}{7} \delta^7 \right) \left( \frac{1}{5} \delta^5 - \frac{2}{7} \delta^7 + \frac{1}{9} \delta^9 \right) q_{02} = 0. \quad (23. 3)
\end{aligned}$$

The values of  $b_{00}$ ,  $b_{02}$ ,  $b_{20}$ , and  $b_{22}$  can be obtained from equations (22. 1) to (22. 4); for  $\mu=0.3$ , the values are:

$$\left. \begin{aligned}
b_{00} &= 0.405052 a_{00}^2 + 0.307930 a_{00} a_{02} + 0.003310 a_{00} a_{22} + 0.034605 a_{02}^2 + 0.003999 a_{02} a_{22} \\
&\quad + 0.000292 a_{22}^2 \\
b_{02} &= -1.231092 a_{00}^2 - 0.134661 a_{00} a_{02} + 0.077909 a_{00} a_{22} - 0.009763 a_{02}^2 + 0.014283 a_{02} a_{22} \\
&\quad + 0.000674 a_{22}^2 \\
b_{20} &= -0.910273 a_{00}^2 - 0.243842 a_{00} a_{02} + 0.001338 a_{00} a_{22} - 0.015872 a_{02}^2 - 0.003371 a_{02} a_{22} \\
&\quad - 0.000469 a_{22}^2 \\
b_{22} &= 1.946906 a_{00}^2 - 0.190484 a_{00} a_{02} - 0.059594 a_{00} a_{22} - 0.056702 a_{02}^2 - 0.005404 a_{02} a_{22} \\
&\quad + 0.000294 a_{22}^2
\end{aligned} \right\} (24)$$

When the expressions (24) for  $b$ 's are introduced in (23.1) to (23.3), we have the three equations containing  $a_{00}$ ,  $a_{02}$ ,  $a_{22}$ ,  $q_{00}$  and  $q_{02}$ . If we put  $a_{00}$  and  $q_{02}$  equal to certain values, the remaining constant values  $a_{02}$ ,  $a_{22}$  and  $q_{00}$  will be determined. Then we may consider  $a_{02}$ ,  $a_{22}$  and  $q_{00}$  as functions of  $a_{00}$  and  $q_{02}$ . Next, we shall discuss several important cases of the problems.

### I. Uniform load $q_{02}=0$ .

We shall consider the case in which the uniform load  $q_{00}$  is on the square part  $-\delta \leq x \leq \delta$ ,  $-\delta \leq y \leq \delta$  of the plate.

We may consider the cases  $\delta=1, 0.8, 0.6, 0.4, 0.2$  and  $0$ .

#### Case I. $\delta=1$ .

When the uniform load is on the whole plate, the calculated values of  $a_{02}$ ,  $a_{22}$ ,  $q_{00}$ ,  $b_{00}$ ,  $b_{02}$ ,  $b_{20}$  and  $b_{22}$  for various assumed values of  $a_{00}$  are given in Table I. I.

Table I. I,  $\delta=1.0$  for  $\mu=0.3$

Values of  $q_{00}$  and displacement constants for  $\mu=0.3$  and various  $a_{00}$  values

$a_{00}$	$a_{02}$	$a_{22}$	$q_{00}$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
0.4	0.125125	0.142953	21.5061	0.081028	-0.199143	-0.158090	0.297585
0.8	0.349623	0.412522	53.5206	0.351310	-0.798871	-0.652841	1.16542
1.0	0.513210	0.642803	76.7848	0.575767	-1.24770	-1.04004	1.79425
1.2	0.713210	0.956489	106.932	0.869939	-1.79271	-1.52778	2.54084
1.6	1.18402	1.86931	194.343	1.68857	-3.15340	-2.81959	4.35456
2.0	1.72965	3.14002	329.585	2.83360	-4.84565	-4.54191	6.55893

The ratio  $\frac{w_{n,ax}}{h}$ , that is the ratio of the maximum deflection to the thickness is identically equal to  $a_{00}$ . The values of  $q_{00}$  the dimensionless load parameter, corresponding to various values of  $a_{00}$  are given in Table I. I. and Fig. 2 shows a curve for  $\frac{w_{max}}{h}$  as a function of  $q_{00}$ . Fig. 2 shows that the straight line  $w_{max}/h = 0.0202305 q_{00}$  obtained by the linear theory becomes tangent to this curve at  $q_{00}=0$ .

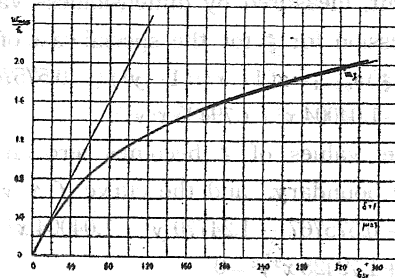


Fig. 2

Table II. I

Ratio of deflection to maximum deflection along  $x$ -axis

$a_{00} \setminus x$	0	0.2	0.4	0.6	0.8	1.0
0.4	1	0.933132	0.740915	0.455726	0.155046	0
0.8	1	0.937711	0.754939	0.474043	0.165849	0
1.0	1	0.940519	0.763539	0.485276	0.172168	0
1.2	1	0.943417	0.772415	0.496868	0.178688	0
1.6	1	0.948880	0.789144	0.518719	0.190980	0
2.0	1	0.953463	0.803179	0.537050	0.201291	0

S. Way has also calculated this case. He used for the expression  $\zeta$  of the follow-

ing one :

$$\zeta = (1-x^2)^2(1-y^2)^2(a_{00} + a_{02}x^2 + a_{02}y^2) . \quad (25)$$

This expression will be identically equal to (17), if we put  $a_{22}=0$ .

For a moderate deflection it was seen the above expression (25) gives a good approximation. When the deflection becomes more and more large, this expression will become less exact and to obtain a better approximation, we must use (17) for the expression of  $\zeta$ . We shall also see later that, in the linear theory, the expression (17) gives a better approximation than Way's one. In order to obtain a more exact solution than Way's one, we must consider the constant  $a_{22}$  and adopt (17) for the expression of  $\zeta$ . When the deflection becomes large, the influence of  $a_{22}$  will becomes important. We see for instance this effect, in our values and Way's in  $q_{00}$  :

$a_{00}$	$q_{00}$ (Way)	$q_{00}$ (our case)
0.4	21.5477	21.5061
1.0	77.1729	76.7848
2.0	335.0131	329.5850

Fig. 2. shows also the comparison between our calculated values and Way's one for  $q_{00}$ . It was seen from Way's paper that the square plate is more rigid than long rectangular plate and more flexible than the circular plate, and these results also hold in our case. The general character of the  $x$ -component of the displacement, measured by dimensionless variable  $\xi$ , can be observed by writting the expression for  $\xi$  for the special case of  $a_{00}=1$  :

$$\left\{ \begin{aligned} (\xi)_{a_{00}=1} &= x(1-x^2)(1-y^2) (-0.575767 - 1.24770y^2 \\ &- 1.04004x^2 + 1.79425x^2y^2). \end{aligned} \right.$$

The values of  $\xi$  becomes zero on  $y$ -axis, the boundary, and the curve  $f(x, y)=0$  :

$$\left\{ \begin{aligned} -0.575767 - 1.24770y^2 - 1.04004x^2 \\ + 1.79425x^2y^2 &= 0 \end{aligned} \right.$$

The curve  $f(x, y)$  is shown in Fig. 3. The values of  $\xi$  for  $y=0$ ,  $a_{00}=1$ , are given in Table III.

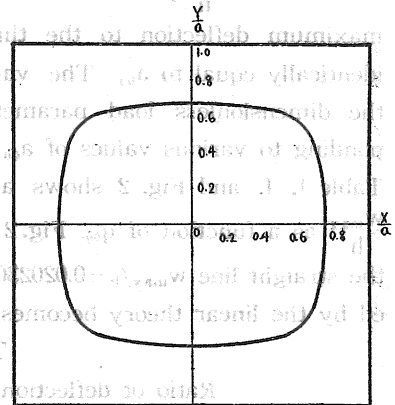


Fig. 3

Table III the values of  $\xi$  for  $y=0$ ,  $a_{00}=1$

$x$	0	0.2	0.37379	0.4	0.6	0.8	0.89019	1.0
$\xi$	0	0.102572	0.138418	0.137545	0.077320	-0.025879	-0.045889	0

We will now consider the calculation of the dimensionless stresses. The membrane stresses at any point are given by

$$\left. \begin{aligned} s'_x &= \left( \xi_x + \frac{\zeta_x^2}{2} \right) + \mu \left( \eta_y + \frac{\zeta_y^2}{2} \right) \\ s'_y &= \left( \eta_y + \frac{\zeta_y^2}{2} \right) + \mu \left( \xi_x + \frac{\zeta_x^2}{2} \right) \\ s'_{xy} &= \eta_x + \zeta_y + \zeta_x \cdot \zeta_y \end{aligned} \right\} (26)$$

The membrane stresses along the  $y$ -axis and the side  $x=1$  are of particular interest. Along the  $y$ -axis:

$$\left\{ \begin{aligned} (s'_x)_{x=0} &= (1-y^2)(b_{00} + b_{02}y^2) + \mu[2b_{20}y^2(1-y^2) + (1-3y^2)(b_{00} + b_{20}y^2) \\ &\quad + 2y^2(1-y^2)^2\{a_{02}(1-y^2) - 2(a_{00} + a_{02}y^2)\}^2] \\ (s'_{xy})_{x=0} &= 0 \\ (s'_y)_{x=0} &= [2b_{20}y^2(1-y^2) + (1-3y^2)(b_{00} + b_{20}y^2) + 2y^2(1-y^2)^2\{a_{02}(1-y^2) \\ &\quad - 2a_{00} + a_{02}y^2\}^2] + \mu(1-y^2)(b_{00} + b_{02}y^2) \end{aligned} \right. (27)$$

These stresses are given in Table IV and plotted in Fig. 4 for  $a_{00}=1$

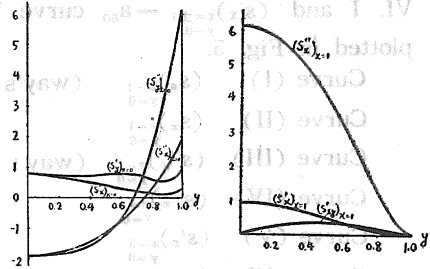


Fig. 4(a) Fig. 4(b)

Table IV. I

(a) The values of  $(s'_x)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.74850	0.74568	0.67491	0.59438	0.50091	0.40014	0.30097	0.20068	0.10498	0.06716	0.27856
2	3.6837	3.5634	3.2656	2.8248	2.3427	1.9071	1.4493	1.2385	0.8877	0.4154	1.0250

(b) The values of  $(s'_y)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.74850	0.74427	0.71840	0.70193	0.71131	0.73358	0.75746	0.72401	0.59319	0.47451	0.92854
2	3.6837	3.5476	3.1981	2.7982	2.5644	2.6665	3.0410	3.2773	3.2512	3.0137	3.4166

Along the side  $x=1$ , the membrane stress are expressed as:

$$\left\{ \begin{aligned} (s'_x)_{x=1} &= -2(1-y^2)\{b_{00} + b_{20} + (b_{02} + b_{22})y^2\} \\ (s'_y)_{x=1} &= \mu(s'_x)_{x=1} \\ (s'_{xy})_{x=1} &= -2y(1-y^2)\{b_{00} + b_{02} + (b_{20} + b_{22})y^2\} \end{aligned} \right. (28)$$

And these values for  $a_{00}=1$ , and 2 are given in Table V. I.

Table V. I

The values of  $(s'_x)_{x=1}$

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	0.92854	0.84943	0.63307	0.34241	0.08243	0
2	3.1417	3.1484	2.4094	1.3972	0.44408	0

The values of  $(s'_{xy})_{x=1}$

$a_{00} \backslash y$	0	0.2	0.4	0.6	0.8	1.0
1	0	0.24617	0.36997	0.30698	0.15639	0
2	0	0.7416	1.1352	0.9876	0.4154	0

The membrane stresses at the center of the plate and at the middle point of the sides are

$$(s'_x)_{x=0} = (s'_y)_{x=0} = b_{00}(1 + \mu) \quad (29)$$

$$(s'_x)_{x=\pm 1} = (s'_y)_{x=\pm 1} = -2(b_{00} + b_{20}) \quad (30)$$

The values of these stresses corresponding to various values of  $a_{00}$  are given in Table VI. I and  $(s'_x)_{x=\pm 1} - a_{00}$  curve has been plotted in Fig. 5.

- Curve (I)  $(s_x)_{x=1} \quad (way's \ curve)$
- Curve (II)  $(s_x)_{x=1} \quad (way's \ curve)$
- Curve (III)  $(s'_x)_{x=1} \quad (way's \ curve)$
- Curve (IV)  $(s''_x)_{x=1} \quad (way's \ curve)$
- Curve (V)  $(s'_x)_{x=0} \quad (way's \ curve)$
- Curve (VI)  $(s'_x)_{x=0} \quad (way's \ curve)$

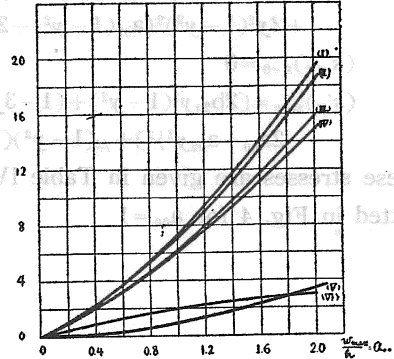


Fig. 5

Table VI. I

$a_{00}$	$q_{00}$	$(s'_x)_{x=0} \quad y=0$		$(s'_x)_{x=1} \quad y=0$	
		Present Case	Way	Present Case	Way
0.4	21.5061	0.10534	0.1070	0.15412	0.1540
0.8	53.5206	0.45451	0.4655	0.60306	0.6023
1.0	76.7848	0.74850	0.7655	0.92854	0.9270
1.2	106.932	1.1309	1.1592	1.3157	1.3130
1.6	194.343	2.1951	2.2647	2.2620	2.2542
2.0	329.585	3.6837	3.8247	3.4166	3.4062

In this Table Way's values are also given for comparison.

The dimensionless bending stresses on the upper surface  $Z = -\frac{h}{2}$  are given by:

$$s'_x = \frac{1}{2}(\zeta_{xx} + \mu\zeta_{yy}); \quad s'_y = \frac{1}{2}(\zeta_{yy} + \mu\zeta_{xx}); \quad s'_{xy} = \zeta_{xy} \quad (31)$$

The values of these stresses along the  $y$ -axis and the side  $x = \pm 1$  are as follows:

$$\left. \begin{aligned} (s''_y)_{x=0} &= (1 - y^2)^2 \{ (a_{02} - 2a_{00}) + (a_{22} - 2a_{02})y^2 \} + \mu \{ (6y^2 - 2)a_{00} + (15y^4 - 12y^2 + 1)a_{02} \} \\ (s''_y)_{x=0} &= \{ (6y^2 - 2)a_{00} + (15y^4 - 12y^2 + 1)a_{02} \} + \mu(1 - y^2)^2 \{ (a_{02} - 2a_{00}) + (a_{22} - 2a_{02})y^2 \} \\ (s''_{xy}) &= 0 \end{aligned} \right\} (32)$$

$$\left. \begin{aligned} (s''_y)_{x=\pm 1} &= 4(1 - y^2)^2 \{ (a_{00} + a_{02}) + (a_{02} + a_{22})y^2 \} \\ (s''_y)_{x=\pm 1} &= \mu(s''_x)_{x=\pm 1} \\ (s''_{xy})_{x=\pm 1} &= 0 \end{aligned} \right\} (33)$$

The values of these stresses corresponding  $y = 0, 0.2, \dots, 1.0$  for  $a_{00} = 1$ , and 2

are given in Tables VII. 1 and VIII. 1.

Table VII. 1

The values of  $(s''_x)_{x=0}$ 

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-1.9328	-1.9072	-1.8286	-1.6913	-1.4869	-1.2039	-0.8294	-0.3750	0.2450	0.9658	1.8158
2	-2.9528	-2.9361	-2.8789	-2.7590	-2.5404	-2.1733	-1.5947	-0.7302	0.5053	2.2063	4.4744

The values of  $(s''_y)_{x=0}$ 

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-1.9328	-1.9259	-1.8961	-1.8166	-1.6428	-1.3124	-0.7458	-0.1532	1.4976	3.4161	6.0528
2	-2.9528	-3.0250	-3.2111	-3.4196	-3.4981	-3.2334	-2.3518	-0.5194	2.6572	7.6313	14.915

Table VIII. I

The values of  $(s''_x)_{x=\pm 1}$ 

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	6.0528	5.9771	5.7488	5.3570	4.7939	4.0550	3.1612	2.1637	1.1680	0.3537	0
2	14.915	14.809	14.472	13.802	12.722	11.128	8.980	6.361	3.548	1.108	0

The bending stresses at the center of the plate and at the middle of a side are:

$$(s''_x)_{x=0} = (s''_y)_{x=0} = (1 + \mu)(a_{02} - 2a_{00}) \quad (34)$$

$$(s''_x)_{x=\pm 1} = (s''_y)_{x=\pm 1} = 4(a_{00} + a_{02}) \quad (35)$$

The values of these stresses corresponding to various values of  $a_{00}$  are given in Table IX. I and have been plotted in Fig. 5. for the case of  $\mu=0.3$ .

Table IX. I

Bending stresses at center and at middle of side

$a_{00}$	$q_{00}$	$(s''_x)_{x=0}$ $y=0$		$(s''_x)_{x=\pm 1}$ $y=0$	
		Present Case	Way's value	Present Case	Way's value
0.4	21.5061	-0.87864	-0.8623	2.1005	2.1469
0.8	53.5206	-1.6255	-1.5848	4.5985	4.7237
1.0	76.7848	-1.9328	-1.8716	6.0528	6.2411
1.2	106.932	-2.1968	-2.1088	7.6408	7.9113
1.6	194.343	-2.6208	-2.4564	11.1361	11.642
2.0	329.585	-2.9528	-2.6838	14.915	15.742

In this Table, Way's values are given for comparison.

From Table II. I, VI. I and IX. I, we see that the discrepancy between our solution and Way's one becomes more and more large when the deflection increases.

The Total stresses at middle of edge are given by

$$(s_x)_{\substack{x=1 \\ y=0}} = (s'_x)_{\substack{x=1 \\ y=0}} + (s''_x)_{\substack{x=1 \\ y=0}}$$

The values of these stresses for various values of  $a_{00}$  are given in Table X. I.

Table X. I

$a_{00}$	0.4	0.8	1.0	1.2	1.6	2.0
$(s_x)_{\substack{x=1 \\ y=0}}$	2.2546	5.2016	6.9813	8.9565	13.398	18.332
Way's value	2.3009	5.3260	7.1681	9.2242	13.896	16.148

The values of  $(s_x)_{\substack{x=1 \\ y=0}}$ ,  $(s''_x)_{\substack{x=1 \\ y=0}}$  and  $(s'_x)_{\substack{x=0 \\ y=0}}$  against  $a_{00}$  are plotted in Fig. 5. Way's values for  $(s_x)_{\substack{x=1 \\ y=0}}$  and  $(s''_x)_{\substack{x=1 \\ y=0}}$  have been also plotted in this figure for comparison.

We now consider the linear theory. In this case we may take  $\xi = \eta = 0$ . The constants  $a_{00}$ ,  $a_{02}$  and  $a_{22}$  are then found from the equations

$$\frac{\partial A}{\partial a_{00}} = 0; \quad \frac{\partial A}{\partial a_{02}} = 0; \quad \frac{\partial A}{\partial a_{22}} = 0.$$

These equations give the values

$$\left. \begin{aligned} a_{00} &= 0.0202305 q_{00} \\ a_{02} &= 0.0053485 q_{00} \\ a_{22} &= 0.000002532 q_{00} \end{aligned} \right\} (36)$$

The approximate expression for the deflection in dimensionless form is therefore

$$\zeta = (1-x^2)^2(1-y^2)^2\{0.0202305 + 0.0053485(x^2+y^2) + 0.000002532 x^2 y^2\} q_{00}.$$

From this expression,  $\frac{W_{\max}}{h}$ ,  $m_x$  at center,  $m_x$  at the middle of side  $x=1$ , and  $Q_x$  at the middle of the side  $x=1$  can easily be calculated by (1) and (3).

These values are given in Table XI, and the values found by Hencky and Way are also given for comparison.

Table XI

	Present case	Hencky	Way
$\frac{W_{\max}}{h}$	0.0202305 $\frac{pa^4}{Dh}$	0.020243 $\frac{pa^4}{Dh}$	0.020202 $\frac{pa^4}{Dh}$
$(m_x)_{\substack{x=a \\ y=0}}$	-0.204632 $pa^2$	-0.2052 $pa^2$	-0.208482 $pa^2$
$(m_x)_{\substack{x=0 \\ y=0}}$	0.0912925 $pa^2$	0.092 $pa^2$	0.092524 $pa^2$
$(Q_x)_{\substack{x=a \\ y=0}}$	-0.870724 $pa$	-0.88 $pa$	-0.9066 $pa$

The reason of the better agreement found here between the present and exact solutions than Way's solution are due to use the expression (17) for the deflection in place of (26). Thus the better approximate solution was found.

Case II.  $\delta = 0.8$

This is the case in which the lateral load is on the square  $-0.8 \leq x \leq 0.8$ ,  $-0.8 \leq y \leq 0.8$ .

We can treat this case as the same method as Case I. The following results are obtained:

Table I. II.

Values of  $q_{00}$  and displacement constants for  $\mu=0.3$  and various  $a_{00}$  values

$a_{00}$	$a_{02}$	$a_{04}$	$q_{00}$	$b_{00}$	$b_{02}$	$b_{04}$	$b_{06}$
0.4	0.110974	0.059023	22.0727	0.079009	-0.201138	-0.156656	0.300910
0.8	0.310756	0.210742	54.8445	0.339961	-0.808219	-0.644744	1.18280
1.0	0.462284	0.361033	78.6988	0.556694	-1.26491	-1.02653	1.82441
1.2	0.645927	0.573143	110.667	0.840246	-1.82213	-1.50690	2.58935
1.6	1.09109	1.25353	198.334	1.62826	-3.22145	-2.77754	4.45759
2.0	1.60474	2.23886	334.920	2.72827	-4.97816	-4.47301	6.74546

Table II. II.

The values of  $w/h$  for various values of  $a_{00}$  along  $x$ -axis

$a_{00} \setminus x$	0	0.2	0.4	0.6	0.8	1.0
0.4	1	0.931827	0.736921	0.45051	0.152612	0
0.8	1	0.935953	0.749454	0.466879	0.161819	0
1.0	1	0.938642	0.757790	0.477767	0.167947	0
1.2	1	0.941443	0.766369	0.489372	0.174246	0
1.6	1	0.946739	0.782587	0.510155	0.186162	0
2.0	1	0.951179	0.796184	0.527914	0.196152	0

Table IV. II.

The values of  $(s'_x)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.72370	0.70567	0.72663	0.57637	0.48438	0.38699	0.28556	0.28136	0.15963	0.15744	0.28190
2	3.5468	3.4412	3.1461	2.7223	2.2544	1.8231	1.4620	1.1295	1.8378	0.4183	1.0468

The values of  $(s'_y)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.72370	0.71847	0.72732	0.69846	0.71184	0.74292	0.75514	0.73493	0.50401	0.49484	0.93968
2	3.5468	3.4270	3.1221	2.7802	2.5926	2.4016	3.05596	3.3179	3.3156	2.1459	3.4895

Table V. II.

The values of  $(s'_x)_{x=1}$

$a_{00}/y$	0	0.2	0.4	0.6	0.8	1.0
1	0.93968	0.85639	0.62940	0.32722	0.06411	0
2	3.4895	3.2142	2.4561	1.4478	0.45809	0

The values of  $(s'_{xy})_{x=1}$

$a_{00}/y$	0	0.2	0.4	0.6	0.8	1.0
1	0	0.25970	0.39014	0.32332	0.11380	0
2	0	0.82901	1.2676	1.0996	0.4582	0



Table VI. II.

The values of  $(s'_x)_{x=0}$  and  $(s'_x)_{x=1}$   
 $y=0$   $y=0$

$a_{00}$	$(s'_x)_{x=0}$ $y=0$	$(s'_x)_{x=1}$ $y=0$
0.4	0.10270	0.15529
0.8	0.44195	0.60957
1.0	0.72370	0.93968
1.2	1.0923	1.3333
1.6	2.1167	2.2986
2.0	3.5468	3.4695

Table VII. II.

The values of  $(s''_x)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-1.9990	-1.9724	-1.8905	-1.7394	-1.5351	-1.2417	-0.8559	-0.3646	0.2316	0.9385	1.7547
2	-3.1138	-3.0968	-3.0374	-2.9119	-2.6818	-2.2961	-1.6907	-0.7982	0.4548	2.1411	4.3257

The values of  $(s''_y)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-1.9990	-1.9863	-1.9399	-1.8330	-1.6324	-1.2745	-0.6900	0.2082	1.5184	3.3469	5.8491
2	-3.1138	-3.1726	-3.3200	-3.4702	-3.5801	-3.1514	-2.2254	-0.3957	2.7026	7.4853	14.419

Table VIII. II.

The values of  $(s''_x)_{x=\pm 1}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	5.8491	5.7408	5.4209	4.9050	4.2200	3.4058	2.5171	1.6262	0.8263	0.2352	0
2	14.419	14.283	13.855	13.086	11.910	10.273	8.173	5.710	3.144	0.970	0

Table X. II.

Bending stresses at center and at middle of side.

$a_{00}$	$(s''_x)_{x=0}$ $y=0$	$(s''_x)_{x=\pm 1}$ $y=0$
0.4	-0.8957	2.0439
0.8	-1.6760	4.4430
1.0	-1.9990	5.8491
1.2	-2.2803	7.3837
1.6	-2.7416	11.764
2.0	-3.1138	14.419

Table X. II.

The total stresses  $(s_x)_{x=1}$  at middle of edge.  
 $y=0$

$a_{00}$	0.4	0.8	1.0	1.2	1.6	2.0
$(s_x)_{x=1}$ $y=0$	2.1992	5.0526	6.7888	8.7170	14.063	17.789

Case III.  $\delta=0.6$ 

Table I. III

Values of  $q_{00}$  and displacements constants for  $\mu=0.3$  and various  $a_{00}$  values

$a_{00}$	$a_{02}$	$a_{22}$	$q_{00}$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
0.4	0.031770	-0.094596	26.3303	0.008621	-0.201682	-0.145803	0.311301
0.8	0.151623	-0.235278	65.5399	0.296630	-0.819594	-0.612675	1.23304
1.0	0.251363	-0.256369	93.8898	0.483554	-1.28641	-0.972726	1.91109
1.2	0.380016	-0.264451	130.032	0.727242	-1.86170	-1.42440	2.72797
1.6	0.704603	-0.158971	233.749	1.39971	-3.33059	-2.61283	4.75731
2.0	1.08946	+0.125487	380.459	2.33342	-5.20786	-4.19137	7.28958

Table II. III

The values of  $w/h$  for various values of  $a_{00}$  along  $x$ -axis

$a_{00} \setminus x$	0	0.2	0.4	0.6	0.8	1.0
0.4	1	0.924528	0.714567	0.421312	0.136188	0
0.8	1	0.928587	0.726997	0.437547	0.145320	0
1.0	1	0.930866	0.733978	0.446665	0.150449	0
1.2	1	0.933274	0.741352	0.456296	0.155867	0
1.6	1	0.937834	0.755317	0.474536	0.166127	0
2.0	1	0.941681	0.767098	0.489924	0.174782	0

Table IV. III

The values of  $(s'_x)_{x=0}$ 

$a_{00} \setminus x$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.62862	0.91374	0.57115	0.50600	0.42606	0.33626	0.23972	0.14126	0.05633	0.04812	0.07360
2	3.0334	2.9514	2.7199	2.3786	1.9418	1.5709	1.1722	0.7775	0.4023	0.2522	1.1148

The values of  $(s'_y)_{x=0}$ 

$a_{00} \setminus x$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.62862	0.632316	0.64556	0.67146	0.71255	0.75242	0.75939	0.69794	0.55876	0.44225	0.97834
2	3.0334	2.98710	2.8780	2.7814	2.7300	2.8896	3.0171	2.9296	2.4328	1.9171	3.7157

Table V. III

The values of  $(s'_x)_{x=1}$ 

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	0.97834	0.89124	0.65390	0.33829	-0.06435	0
2	3.7159	3.4074	2.5618	1.4189	3.7846	0

The values of  $(s'_{xy})_{x=1}$ 

$a_{00} \backslash y$	0	0.2	0.4	0.6	0.8	1.0
1	0	0.29389	0.43863	0.35716	0.05653	0
2	0	1.0562	1.5985	1.3510	0.5136	0

Table VI. III

$a_{00}$	$(s'_x)_{x=0}$ $y=0$	$(s'_x)_{x=1}$ $y=0$
0.4	0.08821	0.15436
0.8	0.38562	0.63209
1.0	0.62862	0.97834
1.2	0.94541	1.3942
1.6	1.8196	2.4262
2.0	3.0335	3.7157

Table VII. III

The values of  $(s''_x)_{x=0}$ 

$a_{00} \backslash y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-2.2632	-2.2368	-2.1265	-1.9395	-1.6720	-1.3188	-0.8839	-0.3660	0.2220	0.8572	1.5016
2	-3.7837	-3.7486	-3.6362	-3.4257	-3.0847	-2.5732	-1.8487	-0.8726	0.08119	1.9174	3.7073

The values of  $(s'_y)_{x=0}$ 

$a_{00} \backslash y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-2.2632	-2.2348	-2.1151	-1.9010	-1.5706	-1.0937	-0.4343	0.4532	1.6184	3.1163	5.0054
2	-3.7837	-3.7815	-3.7548	-3.6438	-3.3496	-2.7353	-1.6276	0.1815	2.8318	6.8923	12.358

Table VIII. III

The values of  $(s''_x)_{x=\pm 1}$ 

$a_{00} \backslash y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	5.0054	4.9056	4.6123	4.1435	3.5296	2.8128	2.0473	1.2994	0.6470	0.1801	0
2	12.358	12.160	11.568	10.596	9.268	7.635	5.778	3.860	2.005	0.588	0

Table IX. III

Bending stresses at center and at middle of side.

$a_{00}$	$(s''_x)_{x=0}$ $y=0$	$(s''_x)_{x=\pm 1}$ $y=0$
0.4	-0.99870	1.7270
0.8	-1.8829	3.8065
1.0	-2.2632	5.0054
1.2	-2.6260	6.3201
1.6	-3.4440	9.2184
2.0	-3.7837	12.358

Table X. III  
The total stresses  $(s_x)_{x=1}$  at middle of edge.

$a_{00}$	0.4	0.8	1.0	1.2	1.6	2.0
$(s_x)_{x=1}$ $y=0$	1.8814	4.4386	5.9837	7.7144	11.645	16.074

Case 4.  $\delta=0.4$

Table I. IV

Values of  $q_{00}$  and displacement constants for  $\mu=0.3$  and various  $a_{00}$  values

$a_{00}$	$a_{02}$	$a_{22}$	$q_{00}$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
0.4	-0.0708187	-0.0551028	43.4578	0.0562027	-0.194869	-0.138860	0.317909
0.8	-0.0837004	-0.117244	108.454	0.238589	-0.787899	-0.566524	1.26391
1.0	-0.0595218	-0.140950	155.532	0.386419	-1.23396	-0.896042	1.96640
1.2	-0.0150639	-0.151853	215.720	0.577129	-1.78453	-1.30665	2.81783
1.6	-0.123365	-0.090992	385.573	1.09771	-3.18982	-2.37883	4.95436
2.0	0.31079	0.109552	638.640	1.81543	-4.99128	-3.79372	0.765076

Table II. IV.

The values of  $w/h$  for various values of  $a_{00}$  along  $x$ -axis

$a_{00} \setminus x$	0	0.2	0.4	0.6	0.8	1.0
0.4	1	0.915073	0.685612	0.383493	0.114915	0
0.8	1	0.917743	0.693788	0.394172	0.120922	0
1.0	1	0.921177	0.704304	0.407909	0.128649	0
1.2	1	0.921137	0.704183	0.407749	0.128559	0
1.6	1	0.924442	0.714305	0.420969	0.135995	0
2.0	1	0.927317	0.723109	0.432469	0.142464	0

Table IV. IV

The values of  $(s'_x)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.50234	0.49978	0.48876	0.46048	0.40419	0.31290	0.19047	0.05933	-0.02882	-0.05505	0.30578
2	2.3601	2.3230	2.2102	2.0164	1.7349	1.3621	0.8994	0.27770	0.0621	0.1123	1.1870

The values of  $(s'_y)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.502345	0.54250	0.64736	0.77412	0.86761	0.86408	0.74514	0.53319	0.34239	0.36891	1.0192
2	2.3601	2.4416	2.6622	2.9701	3.1921	3.2490	2.9935	2.4090	1.7129	1.6582	3.9565

Table V. IV

The values of  $(s'_x)_{x=1}$

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	1.0192	0.92222	0.65929	0.31482	0.02942	0
2	3.9566	3.5945	2.6100	1.3090	0.2012	0

The values of  $(s'_{xy})_{x=1}$ 

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	0	0.30901	0.45446	0.35498	0.09361	0
2	0	1.1603	1.7195	1.3728	0.4074	0

Table VI. IV

$a_{00}$	$(s'_x)_{x=0}$ $y=0$	$(s'_x)_{x=1}$ $y=0$
0.4	0.07306	0.16531
0.8	0.31017	0.65587
1.0	0.50234	1.0192
1.2	0.75027	1.4590
1.6	1.4270	2.5622
2.0	2.3601	3.9566

Table VII. IV

The values of  $(s'_x)_{x=0}$ 

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-2.6774	-2.6165	-2.4366	-2.1459	-1.7581	-1.2926	-0.7742	-0.2336	0.2928	0.7630	1.1286
2	-4.6968	-4.6951	-4.4178	-3.9658	-3.3351	-2.5462	-1.6187	-0.5798	0.5336	1.6722	2.7722

The values of  $(s''_y)_{x=0}$ 

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-2.6774	-2.5961	-2.3622	-1.9746	-1.4448	-0.7852	-0.01214	0.8546	1.7913	2.7708	3.7619
2	-4.6968	-4.6905	-4.3750	-3.8352	-3.0446	-1.9738	-0.5828	1.1760	3.3576	6.0234	9.2407

Table VIII. IV

The values of  $(s'_x)_{x=\pm 1}$ 

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	2.9600	3.6792	3.4274	3.0555	2.5639	2.0033	1.4226	0.8763	0.4210	0.1124	0
2	10.920	9.7033	8.8569	8.3409	7.5635	6.4974	5.1476	3.5812	1.9643	0.6038	0

Table IX. IV

Bending stresses at center and at middle of side

$a_{00}$	$(s''_x)_{x=0}$ $y=0$	$(s''_x)_{x=\pm 1}$ $y=0$
0.4	-1.1321	1.3168
0.8	-2.1888	2.8652
1.0	-2.6774	3.7619
1.2	-3.1396	4.7398
1.6	-3.6996	6.8935
2.0	-4.7968	9.2407

Table X. IV  
The total stresses  $(s_x)_{x=1}$  at middle of edge.

$a_{00}$	0.4	0.8	1.0	1.2	1.6	2.0
$(s_x)_{x=1}$ $y=0$	1.4821	3.5211	4.7811	6.1988	9.4557	13.1973

Case 5.  $\delta=0.2$

Table I. V

Values of  $q_{00}$  and displacement constants for  $\mu=0.3$  and various  $a_{00}$  values

$a_{00}$	$a_{02}$	$a_{22}$	$q_{00}$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
0.4	-0.160216	0.173846	143.210	0.0460901	-0.183556	-0.130252	0.318272
0.8	-0.285974	0.400639	358.886	0.192265	-0.734447	-0.637803	1.26653
1.0	-0.330415	0.553055	516.620	0.308274	-1.14698	-0.820224	1.97177
1.2	-0.362588	0.741758	716.062	0.455874	-1.64958	-1.20495	2.82754
1.6	-0.395543	1.24773	1280.65	0.852554	-2.91837	-2.18143	4.97992
2.0	-0.400287	1.93212	2117.63	1.39002	-4.52560	-3.44239	7.70603

Table II. V

The values of  $w/h$  for various values of  $a_{00}$  along  $x$ -axis

$a_{00} \setminus x$	0	0.2	0.4	0.6	0.8	1.0
0.4	1	0.906834	0.660381	0.350538	0.096278	0
0.8	1	0.908422	0.665243	0.356889	0.099950	0
1.0	1	0.909697	0.668297	0.360878	0.102194	0
1.2	1	0.910461	0.670488	0.365045	0.104538	0
1.6	1	0.912487	0.677690	0.373147	0.109593	0
2.0	1	0.914222	0.683005	0.380088	0.113000	0

Table IV. V

The values of  $(s'_x)_{x=0}$

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	0.40076	0.42179	0.38758	0.16394	-0.06373	+0.30717
2	1.8070	1.8309	1.6123	0.76056	-0.13721	+1.2314

The values of  $(s'_y)_{x=0}$

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	0.40076	0.64188	0.96804	0.74959	0.25255	+1.0239
2	1.8070	2.5825	3.6776	2.9958	1.1876	+4.1047

Table V. V

The Values of  $(s'_x)_{x=1}$

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	1.0239	0.91987	0.63837	0.27523	-0.01146	0
2	4.1047	3.6974	2.5931	1.1615	0.0122	0

The values of  $(s'_{xy})_{x=1}$

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	0	0.30438	0.43980	0.32575	0.05858	0
2	0	1.1386	1.6487	1.2293	0.2343	0

Table VI. V

$a_{00}$	$(s'_x)_{x=0}$ $y=0$	$(s'_x)_{x=1}$ $y=0$
0.4	0.05992	0.16832
0.8	0.24994	0.89108
1.0	0.40076	1.0239
1.2	0.59264	1.4982
1.6	1.1083	2.6578
2.0	1.8070	4.1048

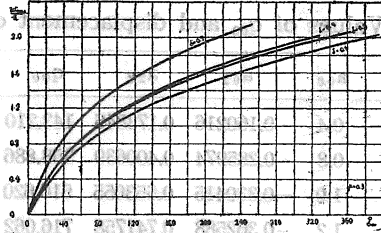


Fig. 6

Table VII. V

The values of  $(s''_x)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-3.0295	-2.9415	-2.6517	-2.2815	-1.7596	-1.1848	-0.5911	-0.0427	0.4038	0.6982	0.8035
2	-5.7204	-5.5558	-5.0759	-4.3212	-3.3560	-2.2633	-1.1382	-0.0787	0.8248	1.5024	1.9196

The values of  $(s''_y)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-3.0295	-2.9129	-2.5607	-2.0255	-1.3131	0.1831	0.3820	1.2270	1.9567	2.4749	2.6783
2	-5.7204	-5.5186	-4.9241	-3.9687	-2.7044	-1.2020	0.4510	2.1529	3.7915	5.2474	6.3988

Table VIII. V

The values of  $(s''_x)_{x=\pm 1}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	2.6783	2.6338	2.4919	2.2842	1.9902	1.6316	1.2282	0.8100	0.4209	0.1227	0
2	6.3988	6.3316	6.1231	5.6703	5.2068	4.4610	3.5245	2.4453	1.3375	0.4102	0

Table IX. V

Bending stresses at center and at middle of side

$a_{00}$	$(s''_x)_{x=0}$ $y=0$	$(s''_x)_{x=\pm 1}$ $y=0$
0.4	-1.2483	0.95914
0.8	-2.4518	2.0561
1.0	-3.0295	2.6783
1.2	-3.5914	3.3497
1.6	-4.6742	4.8178
2.0	-5.7204	6.3988

Table X V  
The total stresses  $(s_x)_{x=1}$  at middle of edge

$a_{00}$	0.4	0.8	1.0	1.2	1.6	2.0
$(s_x)_{x=1}$ $y=0$	1.1274	2.9472	3.7022	4.8479	7.4756	10.504

Case 6  $\delta \rightarrow 0$ . Concentrated load.

If the value of  $q_{00}$  is finite, the terms containing  $q_{00}$  in equations (23) vanish and this case becomes a trivial case and we does not consider it. But we suppose that the value of  $q_{00}$  becomes more and more large as  $\delta$  decreases and the product  $q_{00}\delta^2$  has a limiting finite value when  $\delta$  tends to zero. We write for brevity  $Q = 4 \lim_{\delta \rightarrow 0} q_{00}\delta^2$ .

This is the case in which the concentrated load  $Q$  acts its center of the square plate.

The constants  $a_{02}$ ,  $a_{22}$ ,  $Q$ ,  $b_{00}$ ,  $b_{02}$ ,  $b_{20}$ , and  $b_{22}$  for various assumed values of  $a_{00}$  are given in Table I. VI.

Table I. VI

Values of  $Q$  and displacement constants for  $\mu=0.3$  and various  $a_{00}$  values

$a_{00}$	$a_{02}$	$a_{22}$	$Q$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
0.4	-0.194836	0.295326	21.4598	0.0423101	-0.178411	-0.126933	0.317495
0.6	-0.284121	0.471804	35.6336	0.0965846	-0.400735	-0.286684	0.712701
0.8	-0.366339	0.686732	53.8767	0.174582	-0.710217	-0.511880	1.26299
1.0	-0.437056	0.936225	77.4992	0.278797	-1.10641	-0.804511	1.96600
1.2	-0.501473	1.24481	107.719	0.409576	-1.58568	-1.16467	2.81872
1.4	-0.555493	1.60397	145.790	0.569727	-2.14727	-1.59460	3.81832
1.6	-0.604451	2.02654	192.900	0.758800	-2.78704	-2.09373	4.96218
1.8	-0.648174	2.50795	250.266	0.977917	-3.50300	-2.66289	6.24800
2.0	-0.688229	3.04699	319.094	1.22724	-4.29253	-3.30210	7.67384

The values of dimensionless load  $Q$  corresponding to various values of  $a_{00} = \frac{W_{max}}{h}$  are given in table I. VI and the curve for  $\frac{W_{max}}{h}$  as a function of  $Q$  are plotted in Fig. 7.

Lorentz found that

$$a_{00} = 0.02055 Q.$$

This straight line  $a_{00} = 0.02055 Q$  is also plotted in Fig. 7 for comparison.

We find from this figure that for small deflection there is a good agreement between Lorentz and the present solution, but the discrepancy arises for large deflection. We shall compare later this present solution and the experimental data.

The ratio of deflection to maximum deflection along  $x$ -axis for various values of

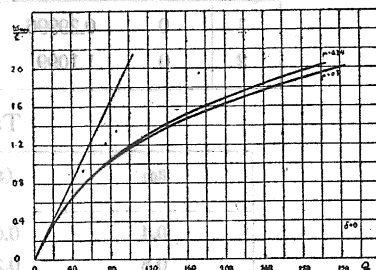


Fig. 7



$a_{00}$  are given in Table II. VI.

The II. VI

Ratio of deflection to maximum deflection along  $x$ -axis

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
0.4	1	0.903644	0.650610	0.337776	0.089199	0
0.8	1	0.908508	0.665505	0.356731	0.100142	0
1.0	1	0.905488	0.656258	0.335154	0.093349	0
1.2	1	0.906195	0.658421	0.347979	0.094938	0
1.6	1	0.907673	0.662950	0.353894	0.098864	0
2.0	1	0.908914	0.666751	0.358857	0.101058	0

Table IV. VI

The values of  $(s'_x)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.36244	0.37368	0.39750	0.40926	0.38008	0.29235	0.15419	0.01113	0.02206	0.00153	0.31543
2	1.5954	1.6285	1.6955	1.7131	1.5831	1.2429	0.8058	0.1468	-0.1950	0.0482	1.2549

The values of  $(s'_y)_{x=0}$

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	0.36244	0.44159	0.64203	0.90545	1.0076	0.96951	0.74597	0.444501	0.25236	0.36093	1.0514
2	1.5954	1.8718	2.5780	3.3893	3.9001	3.7923	3.0306	1.8447	1.0099	1.4572	4.14972

Table V. VI

The values of  $(s'_x)_{x=1}$

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	1.05143	0.94366	0.65214	0.27682	0.01758	0
2	4.1497	3.7252	2.5769	1.0977	-0.0642	0

The values of  $(s'_{xy})_{x=1}$

$a_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1	0	0.29996	0.43127	0.31448	0.04853	0
2	0	1.1099	1.5898	1.1454	0.1540	0

Table VI. VI

$a_{00}$	$(s'_x)_{x=0}$ $y=0$	$(s'_x)_{x=1}$ $y=0$
0.4	0.05500	0.16925
0.8	0.22696	0.67460
1.0	0.36244	1.0514
1.2	0.53242	1.5102
1.6	0.98644	2.6699
2.0	1.5954	4.1497

Table VII. VI

The values of  $(s''_x)_{x=0}$ 

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-3.1692	-3.0684	-2.7786	-2.3266	-1.7569	-1.1270	-0.5029	0.0464	0.4566	0.6759	0.6755
2	-6.0947	-5.8976	-5.3259	-4.4372	-3.3155	-2.0660	-0.8879	0.6723	0.9740	1.4445	1.5741

The values of  $(s''_y)_{x=0}$ 

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	-3.1618	-3.0365	-2.6515	-2.0431	-1.2603	-0.3705	0.5420	1.3778	2.0246	2.3593	2.2518
2	-6.0947	-5.8521	-5.4147	-4.0144	-2.5517	-0.8644	0.8866	2.5632	3.9768	4.9362	5.2471

Table VIII. VI

The values of  $(s''_x)_{x=\pm 1}$ 

$a_{00} \setminus y$	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
1	2.2518	2.2265	2.1488	2.0135	1.8142	1.5474	1.2168	0.8402	0.4574	0.1397	0.6755
2	5.2451	5.2332	5.1817	5.0466	4.7661	4.2772	3.5396	2.0863	1.4590	0.4648	1.5735

Table IX. VI

Bending stresses at center and at middle of side

$a_{00}$	$(s''_x)_{x=0}$ $y=0$	$(s''_x)_{x=\pm 1}$ $y=0$
0.4	-1.2933	0.82066
0.8	-2.5563	1.7346
1.0	-3.1682	2.2518
1.2	-3.7719	2.7941
1.6	-4.9458	3.9822
2.0	-6.0947	5.2451

Table X. VI

The total stresses  $(s_x)_{x=1}$  at middle of edge

$a_{00}$	0.4	0.8	1.0	1.2	1.6	2.0
$(s_x)_{x=1}$ $y=0$	0.98991	2.4092	3.3032	4.3043	6.6521	9.3948

We have hitherto discussed the case of Poisson's ratio  $\mu=0.3$ . Steel have this Poisson's ratio. We will now treat the case of the Poisson's ratio  $\mu=0.34$ , since aluminium has this ratio.

We may consider that these two cases are most important in practice and can now treat the case  $\mu=0.34$  by the same method as the case of  $\mu=0.30$ . The following results are obtained.

Table XI.

Values of  $q_{00}$ ,  $Q$  and displacement constants for  $\mu=0.34$  and various  $a_{00}$  values.

$\delta$	$a_{00}$	$a_{02}$	$a_{22}$	$q_{00}$ or $Q$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
1	0.5	0.16356	0.19400	27.739	0.12371	-0.31444	-0.22435	0.46322
	1.0	0.47326	0.67984	73.807	0.54487	-1.2372	-1.0161	1.8013
	1.5	0.94370	1.7270	157.806	0.95290	-2.7702	-2.0629	4.2591
	2.0	1.5093	3.3754	301.90	2.6166	-4.8338	-4.3784	6.6534
0.8	0.5	0.14094	0.087259	28.450	0.11961	-0.31705	-0.24140	0.46895
	1.0	0.42529	0.38135	75.658	0.52812	-1.2734	-1.0037	1.8313
	1.5	0.87398	1.0899	161.43	1.3164	-2.8537	-2.3525	3.9862
	2.0	1.4269	2.3065	308.18	2.5396	-5.0023	-4.3277	6.8395
0.6	0.5	0.049535	-0.14128	33.995	0.10428	-0.31893	-0.22978	0.48494
	1.0	0.22228	-0.20973	90.341	0.45716	-1.2896	-0.95040	1.9118
	1.5	0.50813	-0.30554	189.86	1.1200	-2.8680	-2.2059	4.1892
	2.0	0.9488	-0.32718	361.03	2.1758	-5.1980	-4.0650	7.3348
0.4	0.5	-0.083953	-0.061941	56.098	0.083762	-0.30560	-0.21309	0.49385
	1.0	-0.077662	-0.0938752	149.51	0.36258	-1.2338	-0.87457	1.9593
	1.5	0.034522	0.011284	315.87	0.88702	-2.7934	-2.0241	4.3559
	2.0	0.21805	0.32708	590.61	1.6887	-4.9700	-3.6848	7.6438
0.2	0.5	-0.19555	0.31067	185.32	0.067836	-0.15001	-0.19914	0.49104
	1.0	-0.32849	0.53644	495.84	0.28933	-1.1510	-0.81190	1.9606
	1.5	-0.38783	1.0658	1049.9	0.69866	-2.5827	-1.8646	4.3660
	2.0	-0.39399	1.8572	1965.6	1.3191	-4.5599	-3.3744	7.6733
0	0.5	-0.24109	0.38199	27.508	0.058167	-0.29110	-0.19090	0.49817
	1.0	-0.44105	0.94297	74.250	0.25781	-1.1068	-0.78390	1.9523
	1.5	-0.59226	1.8338	157.54	0.61187	-2.4584	-1.7879	4.3432
	2.0	-0.70732	3.0671	295.03	1.1399	-4.2994	-3.2172	7.6267

In Fig. 7. the curve for  $\frac{w_{\max}}{h}$  for a concentrated load as a function of  $Q$  are plotted. R. G. Sturm and R. L. Moore carried out the experiments of the deflection of rectangular plates under concentrated load. They used several Aluminium plates, 55" x 55", with different thickness 1",  $\frac{1}{2}$ ",  $\frac{1}{4}$ ", and  $\frac{1}{8}$ ".

The measured values of  $\frac{w_{\max}}{h}$  have been also plotted. From this figure, we find that our present solutions are have better agreement with the measured values than Lorentz's solution. But there are moderate differences measured and the calculated values, if the deflection becomes large. It seems to us that this discrepancy reduces by using more undetermined constants in the assumed expressions for the displacements.

Another method of reducing this discrepancy is to make very careful experi-

ments. Particular care would have to be taken to fulfill the boundary conditions with the experimental plate. It is the boundary conditions in this case that the edges of the plate are all clamped. These conditions are  $u=v=w=0$  at the edges and  $\frac{\partial w}{\partial X}$  at  $X=\pm a$ , and  $\frac{\partial w}{\partial Y}$  at  $Y=\pm a$ . And it is easy to fulfill these conditions for small deflection, but it seen to be very difficult to realize them as the deflection increases, and cause a moderate discrepancy between the present solution and the experimental data.

The ratios of deflection to maximum deflection along  $x$ -axis for various values of  $a_{00}$  and  $\delta$  are given in Table XII.

Table XII  
Ratio of deflection to maximum deflection along  $x$ -axis

$\delta \backslash a_{00}$		0	0.2	0.4	0.6	0.8	1.0
1	1	1	0.93905	0.75903	0.41939	0.16885	0
	2	1	0.94942	0.79080	0.52088	0.19219	0
0.8	1	1	0.93728	0.75361	0.41231	0.16488	0
	2	1	0.94790	0.78615	0.5480	0.18878	0
0.6	1	1	0.92979	0.73070	0.44238	0.14804	0
	2	1	0.93911	0.75922	0.47963	0.16899	0
0.4	1	1	0.91874	0.69683	0.39815	0.12316	0
	2	1	0.92562	0.71791	0.42568	0.13864	0
0.2	1	1	0.90949	0.66952	0.36116	0.10236	0
	2	1	0.91434	0.68336	0.38055	0.11326	0
0	1	1	0.90534	0.65581	0.34456	0.09302	0
	2	1	0.90856	0.66567	0.35745	0.10027	0

Table XIII  
The values of  $(s'_x)_{x=0}$  for  $a_{00}=1$

$\delta \backslash y$	0	0.2	0.4	0.6	0.8	1.0
1.0	0.73013	0.66276	0.61576	0.31609	0.21410	0.30342
0.8	0.70768	0.64075	0.50420	0.29767	0.19516	0.32341
0.6	0.61259	0.57156	0.45038	0.25099	0.13862	0.33541
0.4	0.48586	0.52855	0.41778	0.20805	0.07608	0.34815
0.2	0.38770	0.42060	0.40462	0.18487	0.03566	0.35533
0	0.34547	0.39524	0.40112	0.17489	0.00622	0.35775

The values of  $(s'_y)_{x=0}$  for  $a_{00}=1$

$\delta \backslash y$	0	0.2	0.4	0.6	0.8	1.0
1.0	0.73013	0.71225	0.68984	0.76407	0.61474	0.8924
0.8	0.70768	0.69295	0.77419	0.75947	0.58924	0.9512
0.6	0.61259	0.66828	0.77660	0.75003	0.49587	0.9865
0.4	0.48586	0.77238	0.86807	0.74774	0.37519	1.0240
0.2	0.38770	0.62954	0.95940	0.75195	0.29472	1.0451
0	0.34547	0.62925	1.0034	0.74853	0.26078	1.0522

Table XIV. The values of  $(s'_x)_{x=1}$  for  $a_{00}=1$

$\delta \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1.0	0.9424	0.8614	0.6400	0.3432	0.07933	0
0.8	0.9512	0.8703	0.6490	0.3517	0.08535	0
0.6	0.9865	0.8992	0.6614	0.3446	0.06842	0
0.4	1.024	0.9272	0.6650	0.3210	0.03429	0
0.2	1.045	0.9412	0.6598	0.2702	-0.01121	0
0	1.052	0.9452	0.6566	0.2838	-0.01082	0

The values of  $(s'_{xy})_{x=1}$

$\delta \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1.0	0	0.2538	0.3808	0.3146	0.1093	0
0.8	0	0.2735	0.4122	0.3436	0.1242	0
0.6	0	0.3049	0.4560	0.3735	0.1250	0
0.4	0	0.3179	0.4688	0.3692	0.1020	0
0.2	0	0.3132	0.4556	0.3442	0.0740	0
0	0	0.3081	0.4450	0.3290	0.0583	0

The values of  $(s'_x)_{x=0} = (s'_y)_{x=0}$   
 $y=0$

$\delta \setminus y$	0	0.2	0.4	0.6	0.8	1.0
0.5	0.1658	0.1603	0.1397	0.1122	0.0909	0.0780
1.0	0.7301	0.7077	0.6126	0.4859	0.3877	0.3455
1.5	1.277	1.794	1.501	1.187	0.9362	0.8199
2.0	3.506	3.403	2.916	2.263	1.768	1.528

The values of  $(s'_x)_{x=1} = (s'_y)_{x=0}$   
 $y=1$

$a_{00} \setminus \delta$	0	0.2	0.4	0.6	0.8	1.0
0.5	0.2013	0.2436	0.2510	0.2587	0.2626	0.2655
1.0	0.8924	0.9512	0.9865	1.024	1.045	1.052
1.5	2.220	2.072	2.172	2.631	2.332	2.352
2.0	3.524	3.576	3.778	3.992	4.111	4.155

Table XVI.

The values of  $(s''_x)_{x=0}$  for  $a_{00}=1$

$\delta \setminus y$	0	0.2	0.4	0.6	0.8	1.0
1.0	-2.046	-1.928	-1.547	-0.8317	0.1842	2.004
0.8	-2.110	-1.988	-1.595	-0.8586	0.3052	1.938
0.6	-2.381	-2.220	-1.722	-0.8742	0.3004	1.662
0.4	-2.783	-2.525	-1.799	-0.7512	0.3703	1.254
0.2	-3.020	-2.754	-1.802	-0.5697	0.4827	0.9133
0	-3.271	-2.862	-1.790	-0.4700	0.5410	0.7602

The values of  $(s''_y)_{x=0}$  for  $a_{00}=1$ 

$\delta \backslash y$	0	0.2	0.4	0.6	0.8	1.0
1.0	-2.046	-1.974	-1.670	-0.7173	1.114	5.893
0.8	-2.110	-2.028	-1.664	-0.6676	1.530	5.701
0.6	-2.381	-2.204	-1.611	-0.4263	1.624	4.889
0.4	-2.783	-2.452	-1.495	-0.194	1.792	3.689
0.2	-3.024	-2.653	-1.377	0.3475	1.947	2.686
0	-3.271	-2.742	-1.319	0.5184	2.020	2.356

Table XVIII

The values of  $(s''_x)_{x=\pm 1}$  for  $a_{00}=1$ 

$\delta \backslash y$	0	0.2	0.4	0.6	0.8	1.0
1.0	5.893	5.601	4.679	3.094	1.146	0
0.8	5.701	5.372	4.387	2.811	1.006	0
0.6	4.889	4.507	3.455	2.010	0.6378	0
0.4	3.689	3.374	2.526	1.410	0.4212	0
0.2	2.686	2.506	1.989	1.223	0.4171	0
0	2.236	2.134	1.804	1.212	0.4563	0

Table XVII.

The values of  $(s''_x)_{x=0} = (s''_y)_{y=0}$ 

$a_{00} \backslash \delta$	1.0	0.8	0.6	0.4	0.2	0
0.5	-1.121	-1.151	-1.274	-1.452	-1.602	-1.663
1.0	-2.046	-2.110	-2.331	-2.783	-3.020	-3.271
1.5	-2.755	-2.849	-3.339	-3.974	-4.540	-4.814
2.0	-3.338	-3.448	-4.087	-5.068	-5.888	-6.308

The values of  $(s''_x)_{x=\pm 1} = (s''_y)_{y=\pm 1}$ 

$a_{00} \backslash \delta$	1.0	0.8	0.6	0.4	0.2	0
0.5	2.654	2.564	2.198	1.664	1.218	1.036
1.0	5.893	5.701	4.889	3.689	2.686	2.236
1.5	9.772	9.496	8.032	6.138	4.449	3.631
2.0	14.04	13.71	11.80	8.872	6.424	5.171

## II. The case $q = q_{00} + q_{02}(x^2 + y^2)$ and $\delta = 1.0$

We now consider the case the load  $q$  does not constant and is expressed as  $q = q_{00} + q_{02}(x^2 + y^2)$ . It is useful in practice to consider the case. We put Poisson's ratio equal to 0.3. Then by the same method as the previous problem, we obtain the following results. We shall treat four cases,  $q_{02} = \frac{1}{4}q_{00}$ ,  $-\frac{1}{2}q_{00}$ ,  $\frac{3}{4}q_{00}$ , and  $-q_{00}$ .

Table XIX

The values of  $a_{02}$ ,  $a_{22}$ ,  $q_{00}$ ,  $b_{00}$ ,  $b_{02}$ ,  $b_{20}$ , and  $b_{22}$  for various  $a_{00}$ , and  $q_{02}$ .

Case I.  $q_{02} = -\frac{1}{4}q_{00}$

$a_{00}$	$a_{02}$	$a_{22}$	$q_{00}$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
0.5	0.15277	0.11114	33.242	0.13385	-0.31546	-0.25028	0.45666
1.0	0.47675	0.43044	82.093	0.56202	-1.4806	-1.0303	1.8165
1.5	1.0045	1.0593	181.94	1.4201	-2.8430	-2.4275	3.9359
2.0	1.6731	2.4505	351.55	2.7781	-4.9579	-4.5114	6.6773

Case II.  $q_{02} = -\frac{1}{2}q_{00}$

$a_{00}$	$a_{02}$	$a_{22}$	$q_{00}$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
0.5	0.09104	0.13552	20.781	0.11585	-0.30859	-0.23876	0.49150
1.0	0.17330	0.29175	84.867	0.46065	-1.2312	-0.95283	1.8946
1.5	0.203777	0.95293	181.36	1.0127	-2.6960	-2.1225	4.2122
2.0	0.18834	2.1141	334.76	1.7506	-4.6393	-3.7313	7.4610

Case III.  $q_{02} = -\frac{3}{4}q_{00}$

$a_{00}$	$a_{02}$	$a_{22}$	$q_{00}$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
0.5	0.1141	-0.10975	34.233	0.11910	-0.32022	-0.24176	0.49048
1.0	0.38930	-0.14944	103.20	0.52945	-1.2974	-0.10076	1.8734
1.5	0.85294	0.12045	189.73	1.3315	-2.9338	-2.3718	4.0843
2.0	1.4032	1.0699	401.34	2.5622	-5.1326	-4.3594	7.0061

Case IV.  $q_{02} = -q_{00}$

$a_{00}$	$a_{02}$	$a_{22}$	$q_{00}$	$b_{00}$	$b_{02}$	$b_{20}$	$b_{22}$
0.5	0.074088	-0.18533	37.929	0.11251	-0.32045	-0.23678	0.48709
1.0	0.33086	-0.38883	104.00	0.50896	-1.3087	-0.99284	1.9016
1.5	0.69985	-0.36278	225.02	1.2488	-2.9620	-2.3118	4.1876
2.0	1.1790	-0.10408	431.40	2.3897	-5.2701	-4.2380	7.2727

Table XX

The ratio of deflection to maximum deflection along  $x$ -axis.

Case I.  $q_{02} = -\frac{1}{4}q_{00}$

$a_{00} \setminus x$	0	0.2	0.4	0.6	0.8	1.0
0.5	1.000	0.9328	0.7401	0.4546	0.1529	0
1.0	1.000	0.9392	0.7594	0.4799	0.1691	0
1.5	1.000	0.9463	0.7812	0.5083	0.1851	0
2.0	1.000	0.9525	0.8021	0.5230	0.1990	0

Case II.  $q_{02} = -\frac{1}{2}q_{00}$

$a_{00} \setminus x$	0	0.2	0.4	0.6	0.8	1.0
0.5	1.000	0.9283	0.7261	0.4366	0.1447	0
1.0	1.000	0.9280	0.7252	0.4352	0.1440	0
1.5	1.000	0.9266	0.7209	0.4296	0.1409	0
2.0	1.000	0.9251	0.7163	0.4235	0.1374	0

Case III.  $q_{02} = -\frac{3}{4}q_{00}$

$a_{00} \setminus x$	0	0.2	0.4	0.6	0.8	1.0
0.5	1.000	0.9300	0.7313	0.4433	0.1486	0
1.0	1.000	0.9359	0.7506	0.4670	0.1619	0
1.5	1.000	0.9425	0.7656	0.4934	0.1768	0
2.0	1.000	0.9475	0.7848	0.5131	0.1878	0

Case IV.  $q_{02} = -q_{00}$

$a_{00} \setminus x$	0	0.2	0.4	0.6	0.8	1.0
0.5	1.000	0.9266	0.7223	0.4315	0.1419	0
1.0	1.000	0.9338	0.7430	0.4584	0.1570	0
1.5	1.000	0.9388	0.7583	0.4784	0.1683	0
2.0	1.000	0.9434	0.7721	0.4965	0.1785	0

Table XXI.

The values of  $(s'_x)_{x=0}$  for  $a_{00}=1$

$q_{02} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
$-\frac{1}{4}q_{00}$	0.7306	0.6517	0.4599	0.2394	0.0416	0.2810
$-\frac{1}{2}q_{00}$	0.5989	0.5601	0.4400	0.2357	0.0242	0.2963
$-\frac{3}{4}q_{00}$	0.6883	0.6151	0.4654	0.2616	0.0607	0.2869
$-q_{00}$	0.6617	0.6040	0.4525	0.2467	0.0436	0.2903

$(s'_y)_{x=0}$  for  $a_{00}=1$

$q_{02} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
$-\frac{1}{4}q_{00}$	0.7306	0.7081	0.7047	0.7416	0.5597	0.9366
$-\frac{1}{2}q_{00}$	0.5989	0.6689	0.7947	0.7517	0.4380	0.9874
$-\frac{3}{4}q_{00}$	0.6883	0.6931	0.7314	0.7511	0.5310	0.9564
$-q_{00}$	0.6617	0.6835	0.8300	0.8141	0.5042	0.9678



Table XXII  
The values of  $(s'_x)_{x=1}$  for  $a_{00}=1$ .

$q_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
$-\frac{1}{4}q_{00}$	0.9366	0.8733	0.6964	0.4446	0.1824	0
$-\frac{1}{2}q_{00}$	0.9844	0.8940	0.6485	0.3243	0.0487	0
$-\frac{3}{4}q_{00}$	0.9562	0.8737	0.6484	0.3466	0.0788	0
$-q_{00}$	0.9678	0.8835	0.6536	0.3462	0.0752	0

The values of  $(s'_{xy})_{x=1}$  for  $a_{00}=1$

$q_{02} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
$-\frac{1}{4}q_{00}$	0	0.2931	0.4397	0.3629	0.1256	0
$-\frac{1}{2}q_{00}$	0	0.2683	0.4229	0.3504	0.1231	0
$-\frac{3}{4}q_{00}$	0	0.2814	0.4166	0.3314	0.0967	0
$-q_{00}$	0	0.3407	0.5328	0.4881	0.2393	0

Table XX.  
The values of  $(s'_x)_{x=0}$   
 $y=0$

$q_{02} \setminus a_{00}$	0.5	1.0	1.5	2.0
$-\frac{1}{4}q_{00}$	0.1740	0.7306	1.846	3.612
$-\frac{1}{2}q_{00}$	0.1506	0.5989	1.317	2.276
$-\frac{3}{4}q_{00}$	0.1548	0.6883	1.764	3.331
$q_{00}$	0.1463	0.6617	1.623	3.107

The values of  $(s'_x)_{x=1}$   
 $y=0$

$q_{02} \setminus a_{00}$	0.5	1.0	1.5	2.0
$-\frac{1}{4}q_{00}$	0.2329	0.9366	2.015	3.467
$-\frac{1}{2}q_{00}$	0.2458	0.9844	2.220	3.962
$-\frac{3}{4}q_{00}$	0.2454	0.9562	2.080	3.594
$-q_{00}$	0.2486	0.9676	2.1260	3.696

Table XXIV.  
The values of  $(s''_x)_{x=0}$  for  $a_{00}=1$

$q_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
$-\frac{1}{4}q_{00}$	-1.980	-1.873	-1.522	-1.5440	0.2345	1.7721
$-\frac{1}{2}q_{00}$	-2.375	-2.1852	-1.635	-0.7798	0.2828	1.8773
$-\frac{3}{4}q_{00}$	-2.094	-1.983	-1.616	-0.9092	0.2037	1.6671
$-q_{00}$	-2.170	-2.051	-1.662	-0.9272	0.1953	1.5970

The values of  $(s''_y)_{x=0}$  for  $a_{00}=1$

$q_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
$-\frac{1}{4}q_{00}$	-1.980	-1.928	-1.657	-0.915	1.512	5.907
$-\frac{1}{2}q_{00}$	-2.375	-2.172	-1.521	-0.308	1.875	4.693
$-\frac{3}{4}q_{00}$	-2.094	-2.004	-1.621	-0.602	1.456	5.557
$-q_{00}$	-2.170	-2.053	-1.606	-0.547	1.572	5.323

Table XXV.  
The values of  $(s''_x)_{x=\pm 1}$  for  $a_{00}=1$

$q_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
$-\frac{1}{4}q_{00}$	5.907	5.507	4.383	1.918	0.923	0
$-\frac{1}{2}q_{00}$	4.693	4.394	3.522	1.660	0.762	0
$-\frac{3}{4}q_{00}$	5.557	5.157	3.997	1.827	0.800	0
$-q_{00}$	5.323	4.898	3.730	1.622	0.670	0

The values of  $(s''_y)_{x=\pm 1}$  for  $a_{00}=1$

$q_{00} \setminus y$	0	0.2	0.4	0.6	0.8	1.0
$-\frac{1}{4}q_{00}$	1.772	1.652	1.315	0.5753	0.2768	0
$-\frac{1}{2}q_{00}$	1.408	1.318	1.057	0.4982	0.2288	0
$-\frac{3}{4}q_{00}$	1.667	1.547	1.199	0.5482	0.2399	0
$-q_{00}$	1.597	1.469	1.199	0.4867	0.2011	0

Table XXVI.  
The values of  $(s''_x)_{x=0} = (s''_y)_{y=0}$

$q_{00} \setminus a_{00}$	0.5	1.0	1.5	2.0
$-\frac{1}{4}q_{00}$	-1.101	-1.980	-2.594	-3.025
$-\frac{1}{2}q_{00}$	-1.182	-2.378	-3.635	-4.955
$-\frac{3}{4}q_{00}$	-1.151	-2.094	-2.791	-3.372
$-q_{00}$	-1.204	-2.170	-2.990	-3.667

The values of  $(s''_x)_{x=\pm 1}$   $(s''_y)_{x=0}$   
 $y=0$

$q_{02} \backslash a_{02}$	0.5	1.0	1.5	2.0	0
$-\frac{1}{4} q_{00}$	2.611	5.907	10.02	14.69	
$-\frac{1}{2} q_{00}$	2.284	4.693	6.815	8.753	
$-\frac{3}{4} q_{00}$	2.458	5.557	9.412	13.61	
$q_{00}$	2.296	5.323	8.800	12.72	

Table XXVIII.

The values of  $(s_x)_{x=1}$   
 $y=0$

$q_{02} \backslash a_{02}$	0.5	1.0	1.5	2.0	0
$-\frac{1}{4} q_{00}$	2.844	6.844	12.03	18.16	
$-\frac{1}{2} q_{00}$	2.530	5.677	9.035	12.71	
$-\frac{3}{4} q_{00}$	2.703	6.513	11.49	17.20	
$-q_{00}$	2.545	6.291	10.93	16.42	

Summary

The problems treated here are those the square plate with four clamped edges is under a lateral load. The load has firstly constant value on a part of the plate and secondly symmetrical to its center. The deflection of the plate is assumed of the same order as its thickness. The relation between its maximum deflection i. e., the deflection of its center and the load are obtained. The membrane and bending stresses of the plate are also calculated. These results are compared with the experiment in the case of the concentrated load. In conclusion, I wish to thank Prof. Tomotika for his encouragement during this work. This research has been maintained by the Grant in Aid for Fundamental Scientific Research of the Department of Education, and, certain parts of this paper was read at the meeting of the physical society of Japan.

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